DESIGN CRITERIA:

-- THERMAL FACTOR (Ct):....

-- INTERNAL PRESSURE COEFF:

ALL OTHER ROOF CONDITIONS:..

-- SEISMIC RESPONSE COEFF (Cs):...

LIVE LOADS [UNIFORM (PSF) / POINT LOADS (KIPS)]:

- 2. ROOF SNOW LOAD: -- GROUND SNOW LOAD (Pg): -- FLAT ROOF SNOW LOAD (Pf):. ..22 PSF -- SNOW EXPOSURE FACTOR (Ce):.. -- SNOW LOAD IMPORTANCE FACTOR (I):.....1.1
- 3. WIND DESIGN DATA: -- BASIC WIND SPEED (3 SEC GUST):.. -- WIND IMPORTANCE FACTOR (I):. -- WIND EXPOSURE:. PARTIALLY ENCLOSED -- BUILDING ENCLOSURE:

....0.55

...128 PSF

- -- COMPONENTS AND CLADDING WIND PRESSURE: WALL EDGES AND CORNERS:. ..136 PSF ALL OTHER WALL CONDITIONS:. .124 PSF ROOF EDGES:... .156 PSF ROOF CORNERS:. .156 PSF
- 4. EARTHQUAKE DESIGN DATA: -- SEISMIC IMPORTANCE FACTOR (I):... -- BUILDING OCCUPANCY CATEGORY:..... -- MAPPED SPECTRAL RESP ACCEL (Ss / S1):.....0.125 / 0.056 -- SITE CLASS:.. -- SPECTRAL RESPONSE COEFF (Sds / Sd1):......0.133 / 0.089 -- SEISMIC DESIGN CATEGORY: -- SEISMIC FORCE RESISTING SYSTEM:...

-- RESPONSE MODIFICATION FACTOR (R):.....3.0

STRUCTURAL GENERAL NOTES:

-- DESIGN BASE SHEAR:..

-- ANALYSIS PROCEDURE:..

1. DESIGN AND CONSTRUCTION SHALL CONFORM TO THE "INTERNATIONAL BUILDING CODE, 2006 EDITION". REFER TO THE SPECIAL STRUCTURAL INSPECTION NOTES FOR ADDITIONAL REQUIREMENTS.

2. CONTRACTOR TO VERIFY ALL DIMENSIONS, ELEVATIONS AND EXISTING CONDITIONS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT IMMEDIATELY. 3. IF DISCREPANCIES EXIST BETWEEN STRUCTURAL PLANS, ARCHITECTURAL PLANS, OTHER PLANS, OR SPECIFICATIONS, THE CONTRACTOR OR SUBCONTRACTOR SHALL PROVIDE A WRITTEN REQUEST FOR CLARIFICATION FROM THE ARCHITECT AND/OR ENGINEER PRIOR TO PROCEEDING WITH THE WORK

4. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO EXECUTE AND DETERMINE FINAL ERECTION PROCEDURES, SEQUENCING AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES WHATEVER SHORING, SHEETING, TEMPORARY BRACING, GUYING OR TIE DOWNS WHICH MIGHT BE NECESSARY.

5. THE STRUCTURE AND FOUNDATIONS ARE NOT DESIGNED FOR FUTURE

6. FABRICATORS AND SUPPLIERS SHALL CLEARLY NOTE AND HIGHLIGHT CHANGES MADE IN SHOP DRAWINGS, WHICH DO NOT COMPLY WITH THE CONTRACT

7. COLUMNS, BEAMS, JOISTS, OR TRUSSES SHALL NOT BE FIELD CUT OR TRIMMED FOR ANY REASON WITHOUT THE WRITTEN APPROVAL OF THE

8. HOLES, PIPES, SLEEVES, ETC. NOT SHOWN ON THE DRAWINGS MUST BE REVIEWED BY THE ARCHITECT BEFORE PLACEMENT THROUGH STRUCTURAL

9. IF MECHANICAL AND ELECTRICAL EQUIPMENT SIZES, WEIGHTS, OR LOCATIONS DO NOT COINCIDE WITH EQUIPMENT SHOWN ON THE PLANS, COORDINATE ADJUSTMENTS WITH THE ARCHITECT.

10. NO AREA OF THE STRUCTURE SHALL BE LOADED WITH CONSTRUCTION MATERIALS OR EQUIPMENT THAT EXCEEDS FINAL DESIGN CRITERIA.

UNDER SUPPORTING MEMBERS (TYPICAL UNLESS NOTED). 12. FOR DEFERRED SUBMITTALS (EXAMPLES: PREFABRICATED WOOD OR COLD FORMED STEEL JOISTS, PRECAST CONCRETE ELEMENTS, COLD FORMED FRAMING), SHOP DRAWINGS AND CALCULATIONS SEALED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE JURISDICTION OF THE PROJECT SHALL BE FURNISHED TO THE ENGINEER OF RECORD FOR REIVEW.

11. BEAMS, COLUMNS, WALLS AND FOOTING CENTERS SHALL BE CENTERED

EARTHWORK AND FOUNDATIONS:

1. REFERENCE GEOTECHNICAL INVESTIGATION PREPARED BY RED ROCK CONSULTING, LLC DATED MARCH 25, 2013. THE CONTRACTOR SHALL OBTAIN A COPY OF THIS REPORT AND FOLLOW ALL RECOMMENDATIONS WITHIN.

2. DRILLED PIER INFORMATION A. ALLOWABLE END BEARING CAPACITY = 58 KSF B. ALLOWABLE SKIN FRICTION CAPACITY = 6 KSF C. SOCKET INTO SANDSTONE 10'-0" MIN.

3. LATERAL SOIL PRESSURES PER GEOTECH REPORT

4. ALL PERIMETER AND EXTERIOR FOOTINGS SHALL EXTEND AT LEAST 2'-6" BELOW FINAL ADJACENT GRADE. DEEPEN FOOTINGS AS REQUIRED TO PROVIDE THIS MINIMUM BOTTOM OF FOOTING.

5. ALL WALL AND COLUMN FOOTINGS SHALL BEAR INTO NATIVE UNDISTURBED NON-ORGANIC SOILS OR INTO A CONTROLLED, COMPACTED, TESTED ENGINEERED FILL. THE CONTRACTOR SHALL PROVIDE SOILS TESTING SERVICES TO VERIFY SOIL CONDITIONS AND PROVIDE WRITTEN VERIFICATION TO THE ARCHITECT AND ENGINEER. FLOOR SLAB SUBGRADE PREPARATION SHALL BE CONSTRUCTED AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER. NATIVE SOILS MOISTURE EVALUATIONS SHALL BE PERFORMED A FEW DAYS PRIOR TO THE START OF EARTHWORK, THE EXPOSED SUBGRADE IN THE PROPOSED BUILDING AREA SHALL BE DENSIFIED IN PLACE AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER. IF ACTUAL SITE CONDITIONS DO NOT SATISFY THESE REQUIREMENTS, COORDINATE ADJUSTMENTS WITH ARCHITECT/ENGINEER/GEOTECHNICAL ENGINEER

6. SURFACE WATER SHALL NOT BE ALLOWED TO STAND ADJACENT TO OR DRAIN TOWARDS THE FOUNDATION UNDER ANY CIRCUMSTANCES. PAVEMENTS OR GRADED SOILS AT THE PERIMETER OF THE BUILDING, EXCEPT AS REQUIRED AT EXITS OR AS NOTED, SHALL BE SLOPED AWAY AT 5% OR 6" MIN FOR THE FIRST TEN FEET.

7. FOOTINGS MAY BE POURED TO NEAT LINES OF EXCAVATIONS PROVIDING VERTICAL LINES OF EXCAVATIONS CAN BE MAINTAINED DURING CONCRETE

8. FOUNDATION CONTRACTOR TO ENSURE PROPER ANCHOR ROD PROJECTION AND THAT ANCHOR RODS ARE HELD SECURELY IN POSITION PRIOR TO CONCRETE PLACEMENT. STRUCTURAL STEEL COLUMN ANCHOR RODS SHALL BE SET WITH A TEMPLATE

9. FOUNDATION WALL BACKFILL SHALL NOT BE UNBALANCED BY MORE THAN TWO FEET ON EITHER SIDE AT ANY TIME. BASEMENT WALL AND RESTRAINED RETAINING WALL BACKFILL SHALL NOT BE PLACED, UNLESS THE WALL IS ADEQUATELY BRACED. RETAINING WALL AND BASEMENT WALL BACKFILL SHALL BE FREE DRAINING GRANULAR BACKFILL ACCEPTABLE TO THE GEOTECHNICAL ENGINEER.

CONCRETE AND MASONRY REINFORCING STEEL:

1. ALL REINFORCING BARS SHALL MEET ASTM A615 GRADE 60.

2. ALL MESH SHALL MEET ASTM A-185: LAP A MINIMUM OF 8" OR ONE FULL MESH, WHICHEVER IS GREATER.

3. REINFORCING BARS QUANTITIES SHOWN ARE FOR ESTIMATING PURPOSES 4. PROVIDE AND ADDITIONAL ALLOWANCE OF 1% OF THE TOTAL REINFORCING

SHOWN ON THE FINAL DRAWINGS TO BE FABRICATED AND ERECTED DURING THE PROGRESS OF THE WORK AT THE DIRECTION OF THE STRUCTURAL ENGINEER. FOR THE ADDITIONAL REINFORCING ALLOWANCE, INCLUDE BOTH THE COST OF THE REINFORCING AND THE LABOR TO PLACE IT.

5. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE 3/4" CLEAR FOR SLABS, 2" CLEAR FOR FORMED SURFACES AND 3" CLEAR FOR FOOTINGS (TYPICAL UNLESS

6. CONTRACTOR SHALL VERIFY THAT ALL REINFORCEMENT. SLAB DOWELS. INSERTS, SLEEVES AND EMBEDDED ITEMS ARE PROPERLY LOCATED AND RIGIDLY SECURED PRIOR TO CONCRETE PLACEMENT, "WET STICKING" DOWELS WILL NOT

7. REINFORCEMENT SHALL BE DETAILED IN ACCORDANCE WITH THE LATEST A.C.I. DETAILING MANUAL BY A QUALIFIED AND EXPERIENCED FIRM AND PERSON. PLACE AND SUPPORT REINFORCEMENT WITH ACCESSORIES: MAXIMUM SPACING - 48" CENTERS (PLASTIC-TIPPED LEGS FOR EXPOSED SURFACES). USE 3" SBP SUPPORTS

CAST IN PLACE CONCRETE:

- 1. REQUIRED MINIMUM CONCRETE COMPRESSIVE STRENGTHS AT 28 DAYS:
- a. FOOTING, WALL, AND GRADEBEAM CONCRETE..... b. SLAB ON GRADE, BEAM, AND STRUC SLAB ABOVE GRADE...4000 PSI c. COLUMN CONCRETE ...

2. ALL CONCRETE MIX DESIGNS SHALL HAVE WATER TO CEMENT RATIOS LESS THAN 0.50, WITH A MAXIMUM 60/40 FINE TO COARSE AGGREGATE RATIO. CONCRETE MIX DESIGNS THAT DO NOT CONFORM TO THE ABOVE STANDARD AND/OR CONTAIN WATER REDUCING ADMIXTURES SHALL BE SUBMITTED WITH APPROPRIATE TEST DATA PER A.C.I.. ALL CONCRETE SHALL BE IN CONFORMANCE WITH THE LATEST A.C.I. 301 STANDARDS PUBLICATION.

3. EXTERIOR CONCRETE (FLOOR SLABS, WALLS, ETC) SHALL HAVE 6% (PLUS/MINUS 1%) ENTRAINED AIR.

4. CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" (VERIFY WITH ARCHITECT).

NO ALUMINUM SHALL BE EMBEDDED IN ANY CONCRETE. 6. NO CALCIUM CHLORIDE SHALL BE USED IN CONCRETE

7. THE DESIGN, CONSTRUCTION, AND SAFETY OF ALL FORMWORK IS THE RESPONSIBILITY OF THE CONTRACTOR

8. ALL CONCRETE IS REINFORCED UNLESS SPECIFICALLY NOTED AS UNREINFORCED. REINFORCE ALL CONCRETE NOT OTHERWISE SHOWN WITH THE SAME REINFORCING AS SIMILAR SECTIONS OR AREAS.

9. CONSTRUCTION JOINTS IN GRADE BEAMS, CONTINUOUS FOOTINGS, AND WALLS THAT DO NOT CHANGE DIRECTION SHALL BE SPACED NO GREATER THAN 60'-0" INTERMEDIATE CONTROL JOINTS SHALL BE SPACED AT 25'-0" MAX FOR WALLS. CONTROL JOINTS IN WALLS SHALL ALSO BE LOCATED 15'-0" FROM CORNERS AND AT CHANGES IN WALL THICKNESS

10. WHERE FRESH CONCRETE IS DEPOSITED AGAINST HARDENED CONCRETE (GREATER THAN 8 HRS OLD), CLEAN EXISTING SURFACE OF LAITANCE AND FOREIGN MATERIAL AND DAMPEN THE EXISTING SURFACE. IF REQUIRED, ROUGHEN EXISTING CONCRETE TO 1/4" AMPLITUDE.

11. SLABS ON GRADE SHALL BE 4" THICK MINIMUM ON 4" OF GRANULAR FILL. REINF SLAB WITH 6 X 6-W2.9xW2.9 W.W.F. IN UPPER 1/3 OF SLAB THICKNESS. SUPPLY WWF IN SHEETS. AT INTERIOR SLABS, AN 10 MIL VAPOR BARRIER SHALL BE PLACED BETWEEN THE CONCRETE AND GRANULAR BASE AND CARE SHOULD BE TAKEN DURING CURING TO PREVENT SLAB CURLING. THIS NOTE SHALL BE TYPICAL UNLESS NOTED

12. SAW CUT JOINTS OR KEYED CONSTRUCTION JOINTS IN SLABS ON GRADE SHALL BE SPACED TO DIVIDE THE SLAB INTO PANELS NOT TO EXCEED 225 SQUARE FEET. THE LONGER DIMENSION OF EACH PANEL SHALL NOT EXCEED THE SHORTER DIMENSIONS BY MORE THAN 20%. JOINTS SHALL BE LOCATED AT COLUMN CENTERLINES WHERE POSSIBLE. CONTRACTOR SHALL SUBMIT JOINT LAYOUT TO ARCHITECT FOR APPROVAL. REFER TO TYP DETAIL RC-001A.

13. REINFORCEMENT SHALL BE CONTINUOUS AND LAPPED 53 BAR DIAMETERS (2' -6" MIN.) EXCEPT AS NOTED AND PROVIDE CORNER BARS OF SAME SIZE AND SPACING. 14. MINIMUM CONCRETE WALL REINFORCING (WALL 10" OR GREATER) SHALL BE #5 AT 10" CENTERS EACH WAY, EACH FACE

15. MINIMUM REINFORCING AROUND CONCRETE WALL OPENINGS 2'-0" OR GREATER (TYPICAL UNLESS NOTED): 2 - #5, EXTEND REINF 2'-0" PAST OPENINGS. PROVIDE 2-#5 x 4'-0" DIAGONAL BARS AT CORNERS

16. PROVIDE SLEEVES AT ALL SLAB, BEAM, AND COLUMN PENETRATIONS.

1. HOLLOW LOAD BEARING MASONRY UNITS SHALL CONFORM TO ASTM C90, USE TYPE "S" MORTAR, AND HAVE A MINIMUM COMPRESSIVE STRENGTH (F'm) OF 2000 PSI UNLESS

2. USE LOW-LIFT GROUTING PROCEDURES AND PROVIDE GROUT CONFORMING TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE

STRENGTH OF 2800 PSI AT 28 DAYS. 3. PROVIDE STANDARD (9 GAGE) LADDER TYPE HORIZONTAL REINF AT ALTERNATING COURSES (16" OC). PROVIDE PREFAB LADDER CORNERS AND TEES. LAP LADDER REINF 6" MIN. THIS NOTE TYPICAL AT ALL MASONRY WALLS UNLESS NOTED OTHERWISE

4. VERTICAL REINFORCEMENT AT JAMBS OF OPENINGS SHALL EXTEND THE FULL HEIGHT OF THE WALL UNLESS NOTED OTHERWISE. 5. VERTICAL CONTROL JOINTS SHALL BE 3/8" WIDE OR AS SPECIFIED ON THE DRAWINGS AND FULL HEIGHT OF THE WALL AT LOCATIONS SHOWN ON THE ARCHITECTURAL

DRAWINGS. JOINTS SHALL BE SPACED AT A MAXIMUM OF 20'-0". ALL HORIZONTAL JOINT

REINF SHALL BE DISCONTINUOUS AT CONTROL JOINTS. 6. FOR MASONRY VENEERS, PROVIDE "HOOK AND EYE" TIES AT 24" OC HORIZONTALLY AND 16" OC VERTICALLY. ATTACH APPROPRIATELY TO SUBSTRATE WALL.

7. SHORE ALL MASONRY BOND BEAM LINTELS UNTIL GROUT HAS REACHED THE MINIMUM 28 DAY STRENGTH. REFERENCE TYPICAL DETAIL MS-003 FOR ADDITIONAL

8. PROVIDE 8" MINIMUM BEARING FOR ALL STEEL LINTELS BEARING ON MASONRY (UNO) 9. AT BASE OF WALL, PROVIDE DOWELS FROM CONCRETE THAT MATCH THE SIZE AND SPACING OF THE MASONRY WALL VERTICALS.

10. VERTICAL REINFORCEMENT SHALL BE LAPPED 24" FOR #4 BARS, 30" FOR #5 BARS, 43" FOR #6 BARS, OR WITH AN APPROVED MECHANICAL COUPLER. 11. ALL REINFORCING SHALL BE PROPERLY POSITIONED WITH CENTERING AND CAGING DEVICES WITH A MINIMUM GROUT COVERAGE OF 3/4"

12. GROUT SOLID ALL BLOCK CORES CONTAINING VERTICAL BARS, HORIZONTAL BOND BEAMS, HEADED STUDS, AND OR ANCHORS. FOR MASONRY IN CONTACT WITH EARTH, GROUT SOLID ALL MASONRY AND AIR GAP CAVITIES.

INSTALLED PER HILTI SPECIFICATIONS. SUBSTITUTIONS SHALL BE SUBMITTED FOR

13. ALL ANCHORS WHERE NOTED SHALL BE MANUFACTURED BY HILTI, INC. AND

REVIEW AND APPROVAL WITH APPROPRIATE ICBO EVALUATION REPORTS. 14. MASONRY WALLS SHALL BE REINFORCED WITH NOT LESS THAN #5 VERTICALS AT 24" OC AS WELL AS 2-#5 VERTICAL AT CORNERS, EACH SIDE OF OPENINGS, AND EACH SIDE OF CONTROL JOINTS. TERMINATE VERTICALS WITH STD 90 DEGREE HOOK INTO TOP BOND BEAM. THIS NOTE SHALL BE TYPICAL UNLESS NOTED OTHERWISE.

LIGHT GAGE STRUCTURAL STEEL FRAMING NOTES:

FOLLOWING (UNO):

mils (18 GAGE).

1. LIGHT GAGE FRAMING MEMBERS SHALL HAVE THE FOLLOWING MINIMUM MATERIAL PROPERTIES: FY = 33 KSI FOR 18 GA AND LIGHTER MEMBERS, FY = 50 KSI FOR ALL DIAGONAL STRAP BRACING AND FOR 16 GA AND HEAVIER MEMBERS.

2. ALL DESIGN, FABRICATION, AND ERECTION SHALL BE IN CONFORMANCE WITH AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS." LATERAL LOAD DEFLECTION SHALL BE LIMITED TO 1/600 OF THE SPAN AT LOCATIONS LATERALLY SUPPORTING MASONRY AND 1/360 ELSEWHERE. 3. ALL EXTERIOR STUDS OR LOAD BEARING INTERIOR STUDS SHALL BE PER THE

B. OTHER CONDITIONS, 600S162-33 @ 24" OC (20 GAGE) 4. TRACKS SHALL BE SECURELY ANCHORED TO THE SUPPORTING STRUCTURE TO PROPERLY TRANSFER IMPOSED LOADS. MINIMUM GAGE OF TRACKS SHALL BE 43

A. WHERE STUDS BACK UP FULL HT BRICK, 600S162-43 @ 24" OC (18 GAGE)

5. PROVIDE WALL STUD BRIDGING FOR EACH STUD AS RECOMMENDED BY THE MANUFACTURER. MAXIMUM SPACING SHALL BE 4'-0" CENTERS. 6. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENTS TO PERPENDICULAR MEMBER. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL

PROPERLY FASTENED. 7. DO NOT NOTCH, DRILL OR CUT ANY HOLES IN LOAD BEARING STUDS FOR ELECTRICAL OR MECHANICAL EQUIPMENT: USE EXISTING FABRICATED HOLES. 8. ALL WELDING SHALL BE PERFORMED BY WELDERS EXPERIENCED IN LIGHT

GAGE STEEL FRAMING WORK. TOUCH UP ALL WELDS WITH GALVANIZE COATING. 9. SCREWS IN LIGHT GAGE FRAMING SHALL BE INSTALLED WITH MINIUM EDGE DISTANCES OF 1/2" AND MINIMUM SPACING BETWEEN SCREWS OF 3/4". 10. WHERE BACK-TO-BACK STUD COLUMNS ARE USED, ATTACH WITH #10 SCREWS @ 12" OC MAX, UNO.

11. FOR TOP OF EXTERIOR WALL VERTICAL DEFLECTION SYSTEMS, PROVIDE ONE A. DOUBLE TRACK ASSEMBLY WITH 68 MIL (14 GA) OUTSIDE TRACK B. VERTICAL SLIP CONN (DESIGN FOR 500 LB) AT EA STUD.

12. FOR TOP OF INTERIOR WALL VERTICAL DEFLECTION SYSTEMS, PROVIDE ONE A. DOUBLE TRACK ASSEMBLY WITH 43 MIL (18 GA) OUTSIDE TRACK

B. VERTICAL SLIP CONN (DESIGN FOR 300 LB) AT EA STUD. 13. FOR COLD FORMED JOISTS, PROVIDE JOIST BRIDGING PER MANUF RECOMMENDATIONS @ 8'-0" OC MAX.

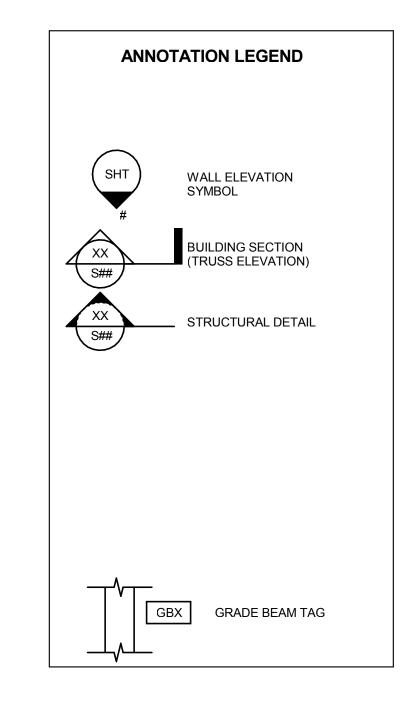
14. ALL STUD WALL TRACKS TO BE FASTENED TO SLABS WITH 0.145" DIA (MIN) PAF @ 16" OC MAX (1" EMBED).

NON-LOAD BEARING LIGHT GAGE STEEL FRAMING NOTES

1. METAL STUD MANUFACTURERS GENERALLY RECOMMEND HORIZONTAL BRIDGING OR STRAPPING TO BE PROPERLY INSTALLED AT 5 FT TO 6 FT OC, MECHANICALLY ATTACHED TO EACH STUD TO PREVENT DAMAGE DURING CONSTRUCTION. EVEN IF ONE SIDE OR BOTH SIDES ARE TO BE SHEATHED WITH RIGID FACING MATERIALS. HOWEVER, AT THE CONTRACTORS RISK AND OPTION, THE RECOMMENDED HORIZONTAL BRIDGING OR STRAPPING MAY BE OMITTED

2. WHEN RIGID FACING MATERIALS ARE NOT ATTACHED TO EITHER SIDE, SUCH AS ABOVE CEILINGS, HORIZONTAL BRIDGING OR STRAPPING AT EACH FACE SHALL BE

3. WHERE THE TOP OF THE STUD WALLS TERMINATE AGAINST PRIMARY STRUCTURAL FRAMING, A "DEFLECTION TRACK" SHOULD BE USED TO ALLOW FOR VERTICAL MOVEMENT. ONE ROW OF THE RECOMMENDED HORIZONTAL BRIDGING SHALL BE PROPERLY INSTALLED BY MECHANICAL ATTACHMENTS TO EACH STUD AS CLOSE TO THE TOP AS POSSIBLE. ANY TEMPORARY SCREWS FROM THE TOP DEFLECTION TRACK TO THE METAL STUDS SHALL BE REMOVED AS SOON AS POSSIBLE TO ALLOW VERTICAL DEFLECTION OF THE PRIMARY FRAMING AND TO PREVENT DAMAGE TO THE STUD WALL. METAL STUDS SHOULD NEVER BE ATTACHED DIRECTLY TO HORIZONTAL STRUCTURAL FRAMING SYSTEMS WITHOUT A DEFLECTION TRACK OR VERTICALLY SLOTTED



STRUC ABREVIATIONS					
+/-	PLUS OR MINUS				
A.R.	ANCHOR ROD				
ADDNL	ADDITIONAL				
ADJ	ADJACENT				
AESS	ARCHITECTURALLY EXPOSED STRUC STL				
AFF	ABOVE FINISH FLOOR				
ALT	ALTERNATE				
ARCH	ARCHITECT OR ARCHITECTURAL				
B/	BOTTOM OF				
B/W	BETWEEN				
BLDG	BUILDING				
BLKG	BLOCKING				
BM	BEAM				
BOT	BOTTOM				
BRG	BEARING				
CF	COLD FORMED				
CHKD	CHECKED				
CIP	CAST IN PLACE				
CJ	CONTROL JOINT				
CJP	COMPLETE JOINT PENETRATION				
CL	CENTERLINE				
CLR	CLEAR				
COL	COLUMN				
CONC	CONCRETE				
CONN	CONNECTION				
CONT	CONTINUOUS				
CTR	CENTER				
db	DIA OF REINF BAR, DIA OF BOLT				
DBA	DEFORMED BAR ANCHOR				
DBL	DOUBLE				
DIA	DIAMETER				
DIAG	DIAGONAL				
DIR	DIRECTION				
DWL	DOWEL				
EA	EACH				
EJ	EXPANSION JOINT				
EL	ELEVATION				
ENGR	ENGINEER				

EDGE OF DECK

STRUC ABREVIATIONS				
EOS	EDGE OF SLAB			
EQ	EQUAL			
EQP	EQUIPMENT			
EW	EACH WAY			
EXIST	EXISTING			
EXT	EXTERIOR			
F.V.	FIELD VERIFY			
FLG	FLANGE			
FLR	FLOOR			
FND	FOUNDATION			
FS	FAR SIDE			
FTG	FOOTING			
G.C.	GENERAL CONTRACTOR			
GA	GAGE			
GALV	GALVANIZED			
GB	GRADE BEAM			
HORIZ	HORIZONTAL			
HSA	HEADED STUD ANCHOR			
HSS	HOLLOW STRUCTURAL SECTION			
IF	INSIDE FACE			
INT	INTERIOR			
JST	JOIST			
k	KIPS (1000 LBS)			
LCE	COMPRESSION EMBEDMENT			
	LENGTH			
LCS	COMPRESSION LAP SPLICE LENGT			
LLH	LONG LEG HORIZONTAL			
LLV	LONG LEG VERTICAL			
LTE	TENSION EMBEDMENT LENGTH			
LTS	TENSION LAP SPLICE LENGTH			
LW	LIGHTWEIGHT			
MAS	MASONRY			
MATL	MATERIAL			
MNFR	MANUFACTURER			
MTL	METAL			
NIC	NOT IN CONTRACT			
NS	NEAR SIDE			
NS	NON-SHRINK			
NTS	NOT TO SCALE			
O.F.	OUTSIDE FACE			

	RUC ABREVIATIONS
OC	ON CENTER
OPP	OPPOSITE
OVS	OVERSIZED
P/C	PRECAST
PAF	POWDER ACTUATED FASTENER
PEN	PENETRATION
PL	PLATE
PLF	POUNDS PER LINEAL FOOT
PMB	PRE-ENGINEERED METAL BUILDII
PREFAB	PREFABRICATED
PRELIM	PRELIMINARY
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
R.F.	RIGID FRAME
RC	REINFORCED CONCRETE
RE: OR REF	REFER TO
REINF	
REQD	REQUIRED
SC	SLIP CRITICAL
SDS	SELF DRILLING SCREW
SIM	SIMILAR
SLV	SHORT LEG VERTICAL
SOG	SLAB ON GRADE
SQ	SQUARE
SS	STAINLESS STEEL
STD	STANDARD
STIR	STIRRUPS
STL	STEEL
SW	SHEAR WALL
SYM	SYMMETRIC
T&B	TOP AND BOTTOM
T/	TOP OF
TOS	TOP OF STEEL
TRANS	TRANSVERSE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W/	WITH
W/O	WITHOUT
WF	WITHOUT WIDE FLANGE
WP WWF	WORK POINT WELDED WIRE FABRIC
VV VV I	WELDED WIRE FABRIC

11.26.12

S-001

REVISIONS	DATE		
00 % Construction Documents	06.28.13		
9 % Construction Documents	02.28.13		
3 % Construction Documents	11.26.12		
3 % Schematic Design Revised	11.02.12		
3 % Schematic Design	08.06.12		
evisions.			

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VED:	APPROVED: Industrial Hygienist APPROVED: Chief of Engineering Service		BUILDING NUMBER	AUTOCAD FILE NAME:	Location				
			623-12-101		Designer	PWS			
VED:	APPROVED: Infection Control Nurse	APPROVED: Chief of Staff	PROJECT NUMBER	CONTRACT NO:	Designed By	Drawn By	Checked By	DRAWING	
VED:	APPROVED: Safety Manager	APPROVED: Associate Director	GENERAL STRUCTURAL NOTES & ABBREVIATIONS		GENERATOR STANDBY SYSTEM			Scale	
					FULL FACILITY				
VED: President A.F.G.E 2250	APPROVED: Energy Engineer	APPROVED: Medical Center Director	DRAWING TITLE:	DRAWING TITLE:			Project Title		

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